## **Reasoned document**

S. N	CI No	Provisional Draft DBR	Suggested Changes	Reason/Justifi cation
1	2.1.3.2.	Prestressing Units	Prestressing Equipments	Specific term used.
2	2.2.3	Fire Resistance Period  All the structural elements in the station building shall be designed for a minimum fire resistance period of 2 hours. The minimum element thicknesses for this fire resistance shall be as per clause 21 of IS:456 for concrete structures and as per Section 16 of IS:800 for steel structures.	Fire Resistance Period  All the structural elements in the station building shall be designed for a minimum fire resistance period of 2 hours. The minimum element thicknesses and other requirements for this fire resistance shall be as per clause 21 of IS:456 for concrete structures and as per Section 16 of IS:800 for steel structures. NBC 2016 shall be followed for fire and line safety requirements.	Reference of NBC 2016 added
3	2.4.1.	Dead Loads  Dead load shall be based on the actual cross section area and unit weights of materials and shall include the weight of the materials that are structural components of Elevated Station and permanent in nature. It shall be calculated in accordance with IS:875 Part 1.	Dead Loads  Dead load shall be based on the actual cross section area and unit weights of materials and shall include the weight of the materials that are structural components of Elevated Station and permanent in nature. It shall be calculated in accordance with IS:875 Part 1. Probable variation in Dead Load shall be calculated as per clause 20.3 of IS: 456.	As per IS 456 clause 20.3
4	2.5.1	Each component of the structure shall be designed and checked for all possible combinations of applied loads and forces. They shall resist effect of the worst combination. Following shall be considered:  (i) Load combinations and factors as per Table 18 of IS: 456 for Plain and Reinforced Concrete Structures.  (ii) Load combination and factors as per Table 7 of IS: 1343 for Pre-stressed Concrete Structures.  (iii) Load combination as per Section 3 and factors as per Section 5 of IS: 800 for Steel	designed and checked for all possible combinations of applied loads and forces. They shall resist the effect of the worst	Specific clause of codes added; Latest nomenclature of Seismic Code added.

		structures.  (iv) Load combination as per clause 6.3 of IS: 1893 (Part-I)  (v) Load combinations as per IRS CBC and RDSO guidelines for Seismic design of Railway Bridges where Metro live loads are applicable.	(iv) Load combination as per clause 6.3 of IS: 1893 (Part-I)  (v) Load combinations as per IRS CBC and Seismic Code for Earthquake Resistant Design of Railway Bridges where Metro live loads are applicable.	
5	2.6.1	Lateral Sway  The lateral sway at the top of the building due to wind loads should not exceed H/500, where H is the height of the building.	Lateral Sway  The lateral sway at the top of the building due to transient wind loads should not exceed H/500, where H is the height of the building.	As per IS 456 clause 20.5.
6	2.1.4	Structural Steel New note	(iv) Self-weight multiplier of 1.10 will be employed for structural steel components of structure to account for the weight of connection plates, angles, bolts and welds to be considered.	B&S remarks
7	2.4.4.1	Drift Limitation The storey drift in the building shall satisfy the drift limitation specified in clause 7.11.1 in IS:1893.	<b>Drift Limitation</b> The storey drift in the building shall satisfy the drift limitation specified in clause 7.11.1 in IS:1893 Part-1.	Code part specified
8	2.1.4	a) Hollow steel sections as per IS:4923-1997	a) Hollow steel sections as per IS:4923.	Year has been removed